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ҚАЗАҚСТАН РЕСПУБЛИКАСЫ  
ҰЛТТЫҚ ҒЫЛЫМ АКАДЕМИЯСЫ  
Satbayev University

# Х А Б А Р Л А Р Ы

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## ИЗВЕСТИЯ

НАЦИОНАЛЬНОЙ АКАДЕМИИ НАУК  
РЕСПУБЛИКИ КАЗАХСТАН  
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**PREDICTION OF THE CUTTING RESISTANCE FORCE OF THE  
SOIL CONTAINING STONY FRACTIONS**

**Abstract.** Soils containing stony inclusions can be represented as a soil-stone mixture with a different content of stony inclusions. This mechanical mixture is mainly a source of generating random loads and premature fatigue destruction of the metal structure of the earthmoving machine during its operation, increasing the cost of soil development. In order to predict the strength of the cutting resistance has been developed of the soil containing a spherical rocky inclusion, an analytical model based on the theory of the ultimate equilibrium of the bulk medium, dimensional analysis of dependence parameters and similarity principles. The principle of dividing the surface of a rocky inclusion is applied into elementary ball retaining belts, to which components of passive ground pressure are applied. Stress zones are conditionally distinguished on the surface of the cutting tool (homogeneous zone), and around the rocky inclusion (rocky zone). In the absence of a rocky inclusion, the resistance force of a homogeneous ground to cutting is calculated. The value of the resistance force of the soil is influenced by the strength parameters and the geometry of the cutting of the soil, the size of the rocky inclusion and the quantitative content of stones in the soil.

The types of soil destruction with rocky inclusion are determined depending on the coordinate of the action of the cutting element. The correctness of the developed model is evaluated in comparison with experimental data. A computational analysis is carried out of the dynamics of the cutting resistance force arising from the heterogeneity and structural randomness of soils containing rocky inclusions. With an increase in the content of stones in the soil, the values of the angle of friction of the soil against the working body and the specific cohesion of the soil increase, which ultimately cause an increase in the strength of resistance of soils containing stony inclusions to cutting.

It should be noted that the geometric shape of stony inclusions in the soil array varies randomly, and the above technique allows us to develop a model for predicting the strength of resistance to cutting of soil containing various stony inclusions, for example ellipsoid shape.

**Key words:** Soils containing stony fractions, stony fractions, cutting resistance force, analytical model, theory of the limiting equilibrium of a loose medium, dimensional analysis, structural state, heterogeneity, structural randomness.

**Introduction.** There is a tendency in the world to automate the process of digging soil with earthmoving machines [1] - [2]. The effective implementation of the plan for the development of heterogeneous soils by earthmoving machines mainly depends on taking into account the ground conditions and power constraints on the working equipment [3]. It is known that soils contain stony fractions in different amounts, their geometric shape and size, and the depth of their occurrence in the soil mass change randomly. Soils containing stony fractions can be represented as a soil-stone mixture with a different content of stony fractions by mass.

The metal structure of the working equipment of earthmoving machines undergoes random loading processes during operation. The structure of these random processes depends mainly on the presence of solid large rock fragments in the ground – stony fractions. The area of stony soils in the Kyrgyz Republic only in the zone of agriculture is about 3809 thousand hectares, including: slightly stony – 1477 thousand hectares; medium stony – 1495 thousand hectares; strongly stony -10.4 thousand hectares [4].

Methods for determining the parameters of both homogeneous soil and soil with rocky inclusions

have been developed to predict the strength of soil resistance, modeling the trajectories of the excavator's working equipment in automatic mode [5] - [7]. The soil containing stony fractions is a type of uneven loose sedimentary material consisting mainly of stone, fine-grained soil, water and pores, which is located between the homogeneous soil and the fractured rock mass of the Quaternary period. The strength and deformation characteristics of the stony fractions are directly affected by its structural state: the number, location, shape, cementation of block stones and porosity. On the basis of three-dimensional modeling, a mesostructural model of a stony fractions was developed and used to model shear strain curves [8]. In [9] the deformation characteristics of the shear zone and the movement of block stones in stony fractions are studied experimentally.

Analytical [10] - [13] and numerical models [14] - [19] have been developed to predict the strength of soil resistance to cutting. In the work [11], the cutting resistance force is determined without taking into account the effects of the angle, cutting width, and features of the normal and tangent components of the passive ground pressure of a homogeneous and rocky zone. The volume of the soil is determined based on the pre-set values of the shear angles of the central and lateral parts of the fracture slot.

Based on the Poisson distribution of random events (components of the drag force) in the half-space of the bulk material Z. Korzen [12] established a mathematical regression dimensionless model of the resistance of inhomogeneous rocks to cutting with a curved cutting tool. To establish the coefficients of the mathematical model, active orthogonal experiments were carried out involving the cutting geometry, humidity, velocity, and average size of mineral rock particles. The regression model does not take into account the cohesive and frictional properties, which are the main factors that affect the strength of the resistance of the soil containing stony fractions to cutting.

The aim of the study is to develop an analytical model for predicting the cutting resistance of soil containing stony fractions.

**Physical characteristics of the dimensional cutting space of soils with stony fractions.** The first necessary step is to determine the variables that significantly affect the soil-cutting element system. The relevant characteristics of the soil are those that have a fairly direct relationship to the forces or movements. According to the generally accepted notation, the set of parameters that affect the process of mechanical cutting of soil containing stony fractions can be expressed as follows:

$$P_{cs} = f(\sigma_n, \tau_n, \gamma, C, \varphi, \varphi_0, p, v, g, \alpha, h, b, R_s), \quad (1)$$

where:  $\sigma_n$  and  $\tau_n$  - normal components of the passive ground pressure,  $\gamma$  - volume weight,  $C$  - adhesion,  $\varphi$  - angle of internal friction,  $\varphi_0$  - angle of external friction,  $p$  - loading,  $v$  - cutting speed,  $g$  - acceleration of gravity,  $\alpha$  - cutting angle,  $h_s$  - cutting depth,  $b_s$  - cutting width,  $R_s$  - radius of the stony fractions.

To satisfy the similarity principles based on the pi-theorem [20] and the ratio of forces to gravity form dimensionless groups of parameters:

$$\frac{P_{cs}}{\gamma L^3} = f\left(\frac{\sigma_n}{\gamma L}, \frac{\tau_n}{\gamma L}, \frac{C}{\gamma L}, \frac{p}{\gamma L}, \frac{v^2}{gL}, \varphi, \varphi_0, \alpha\right). \quad (2)$$

In (2), each group is dimensionless, does not depend on any particular parameter, and to ensure similarity between the parameters of the prototype of the nature -  $Pt_{ni}$  and the model -  $Pt_{mi}$ , the scale value -  $k_{Li}$  is introduced:

$$Pt_{mi} = k_{Li} Pt_{ni} \quad (3)$$

In this case, the process of cutting soil with stony fractions is described by the same differential equations of equilibrium, continuity conditions, stress state equations based on the general provisions of the theory of elasticity and the theory of plasticity. The cutting speed, which varies in the range (observed in production conditions), does not significantly affect the value of the sand cutting force [21]. The determination of the cutting resistance of the soil containing the stony fractions is based on the theory of the limiting equilibrium of a loose medium [22]. The design scheme of the soil resistance acting on the cutting element of the earthmoving machine is shown in Fig. 1, where it is possible to conditionally distinguish the stressed zones of homogeneous soil and the zones around the stony inclusion in compliance with the condition of geometric similarity. Surface of the cutting element (bucket tooth) and stony fractions are represented by a set of elementary retaining walls and to follow them accordingly elemental powers of resistance of the soil

cutting  $dP_o$  homogeneous zones (zone I) and the resistance of the soil around the stony  $dP_s$  (zone II, zone III). The destruction of the soil occurs when the maximum stress state is reached on the sliding surfaces. For elementary retaining walls, to determine the force of resistance to cutting, you can apply the forces of ground resistance [22]. The cutting force of the soil containing the stony fractions is balanced by the forces of the resistance of the soil to cutting:

$$P_{cs} = P_{rs} \tag{4}$$

The soil containing the stony fractions has a complex stress state. Taking into account the many factors that affect the process of cutting the soil, for practical calculations, an approximate determination of the resistance force of the soil containing the stony fractions is carried out.

The  $P_{rs}$  force includes the ground resistance force on the surface of the cutting element  $P_h$  and the resistance force on the surface (around) the rock particle  $P_s$ , as well as the force of pushing the stone to the surface of the  $P_g$

$$P_{rs} = P_h + P_s + P_g \tag{5}$$

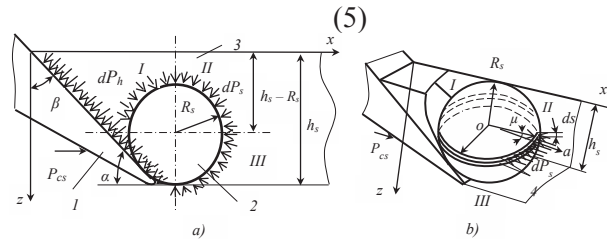


Fig. 1. Calculation scheme for determining the resistance of soil containing spherical stone activation: 1 – cutting element, 2 - stony fraction, 3 – ground, 4 – basic retaining wall (ball belt),  $\beta$  is the angle of inclination of the surface of the cutting element from the vertical,  $h_s$  – depth of the stony fraction,  $ds$  – height retaining walls, I, II, III – coverage of passive earth pressure.

In the absence of a stony fraction, when  $R_s = 0$ , forces  $P_s = 0$ ,  $P_g = 0$ , we have only a homogeneous soil:  $P_{rs} = P_h$ . The resistance force of a homogeneous ground zone to cutting is defined as [23]:

$$P_h = \sqrt{(A - Hh_s / \sin \alpha)^2 + (A \tan \varphi_0)^2} \sin(\alpha + \varphi_0) b_s \eta_{sp_s}, \tag{6}$$

where  $A = a_{\beta_i} (0,5\gamma h_s^2 + ph_s + Hh_s) / \sin \alpha$ ,  $H = C \text{ctg} \varphi$ ,  $a_{\beta_i}$  – the slope coefficient of the retaining walls,  $\eta_{sp_s}$  - the spatiality coefficient.

Depending on  $\beta$ , the elementary ball belts facing the day surface (zone II) will be located in steep, intermediate and flat areas, and the ball belts that face deep into the ground mass (zone III) are located in the so-called broken areas. The coefficient  $a_{\beta_i}$  is defined in [22]. The position of the elementary ball belts determines the rotation of  $R_s$  by an angle  $\mu_1$  with a given step, relative to the  $oa$  axis (Fig. 1b).

The curved surface of the elementary ball belts is approximated by a straight surface, and the approximation error depends on the size of the split step (Fig. 2). The depth of the detrital-stone particle in the soil mass is determined by the expression

$$h_s = h + k_h R_s, \tag{7}$$

where  $k_h$  is the depth coefficient of the stony fraction,  $h$  - cutting depth of homogeneous soil.

Depending on the value of  $k_h$ , the soil can be destroyed with intensive ( $k_h = 0 \dots 0.25$ ), with extensive removal of stone from the massif ( $k_h = 0.25 \dots 1.75$ ), and with the indentation of the stony fraction into the soil massif ( $k_h > 1.75$ ) [24]. In our case,  $k_h = 0$ , then  $h_s = h$ .

Next, we consider the acting forces on the example of one elementary retaining wall (Fig. 2b) with an angle of  $\mu$ .

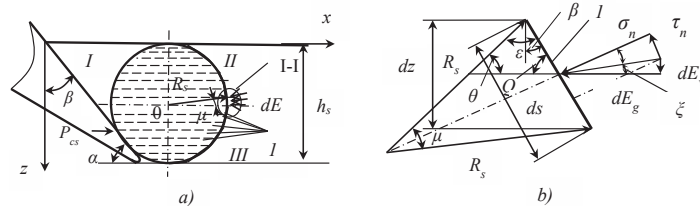


Fig. 2. Scheme for determining the resistance force on the surface of a stony fraction: 1 – elementary retaining wall,  $dE$  – elementary resultant ground pressure,  $dE_v$  - vertical and  $dE_g$  – horizontal components of  $dE$ .



The range of variation of the angle of rotation  $R_s$  from the oa axis towards the day surface and deep into the soil mass is  $0 \dots \pi/2$  with a given step -  $\mu$ . The split step is defined by the expression:

$$\mu = \pi / 2n, \quad (8)$$

where  $n$  is the total number of partitions into elementary strips.

The inclined height is a constant value in all elementary walls, and is determined by the expression:

$$ds = 2R_s \sin \mu / 2 \quad (9)$$

The angle  $\varepsilon$  is determined by the sine theorem:

$$\varepsilon = \sin^{-1}(R_s \sin \mu / ds) \quad (10)$$

The point of intersection of the secant plane with the particle surface determines the position of the elementary retaining strip relative to the oa axis (Fig.1b), characterized by the angle  $\theta_i$ , which is determined by the expression:

$$\theta_i = i\mu, \text{ where } i = 0,1,2,3\dots n \quad (11)$$

moreover  $\theta_0 = 0$ .

Determine the angles  $Q_i$  and  $\beta_i$ :

$$Q_i = \pi - \theta_i - \varepsilon, \text{ where } i = 0,1,2,3\dots n \quad (12)$$

moreover  $Q_0 = \pi/2$ .

$$\beta_i = \pi/2 - Q_i, \text{ where } i = 0,1,2,3\dots n \quad (13)$$

moreover  $\beta_0 = 0$ .

In turn, the height of the elementary strips is determined by the expressions:

in the direction of the day surface of the ground

$$z_{u_i} = (h_s - R_s) - ds \cos \beta_i, \text{ } i = 0,1,2,3\dots n \quad (14)$$

moreover  $z_0 = h_s - R_s$ ;

to the side deep into the ground mass

$$z_{m_i} = (h_s - R_s) + ds \cos \beta_i, \text{ } i = 0,1,2,3\dots n \quad (15)$$

moreover  $z_0 = h_s - R_s$ .

The area of an elementary strip is defined by the following expression:

$$dF_i = dv_{s_i} ds, \text{ where } i = 0,1,2,3\dots n \quad (16)$$

where  $dv_{s_i}$  is the average length of the generators of the elementary strip.

At the time of the limit equilibrium cutting force is balanced by the passive soil pressure applied on the surface of the cutting element (zone I, Fig. 1,2), and passive pressure acting on the surface of stony fractions, viewed in the direction of cutting (zones II, III, Fig. 1,2). In this regard, the expression for determining  $dv_{s_i}$  takes into account part of the length of the generators of the elementary strip:

$$dv_{s_i} = \pi(R_i + R_{i+1})/2, \text{ where } i = 0,1,2,3\dots n \quad (17)$$

$$R_i = R_s \cos \theta_i, \text{ where } i = 0,1,2,3\dots n \quad (18)$$

moreover  $R_n = R_s$ .

Then: 
$$dF_i = \pi R_s^2 \sin \frac{\mu}{2} (\cos \theta_i + \cos \theta_{i+1}), \text{ where } i = 0,1,2,3\dots n. \quad (19)$$

The elementary force of the cutting resistance on the surface of the stony fraction is equal to:

$$dP_s = dE_g dF_i \quad (20)$$

In turn, the tangent of the pressure component to the ground cutting path is defined as:

$$dE_{g_i} = dE_i \cos \xi_i, \quad (21)$$

where  $dE_i$  is the resultant of the passive ground pressure.

The value of the angle  $\xi_i$  is determined by the expressions:

$$\xi_i = \pi/2 - Q_i - \varphi_0, \text{ if } (Q + \varphi_0) < \pi/2 \tag{22}$$

$$\xi_i = Q_i + \varphi_0 - \pi/2, \text{ if } (Q + \varphi_0) > \pi/2. \tag{23}$$

The resultant of the ground pressure is determined by the dependence:

$$dE_i = \sqrt{\sigma_{n_i}^2 + \tau_{n_i}^2}. \tag{24}$$

The normal (as hydrostatic) ground pressure on the elementary strips [22] is determined by the formula:

$$\sigma_{n_i} = (\gamma z_i + p + H) a_{\beta_i} - H, \quad i = 1, 2, 3 \dots n, \tag{25}$$

where,  $z_i$  is the height of elementary spherical strips on the surface of a stony fraction;

and the tangential ground pressure on the elementary strips [22] is determined by the expression:

$$\tau_{n_i} = (\sigma_{n_i} + H) \tan \varphi_0, \quad i = 1, 2, 3 \dots n. \tag{26}$$

The tangential drag force is equal to the sum of the elementary ground cutting resistance forces  $dP_{s_i}$

$$P_s = \sum_{j=1}^{j=4} \sum_{i=1}^{i=n} P_{s_j} dP_{s_i}, \tag{27}$$

where  $j = 1$  is a steep area,  $j = 2$  is an intermediate area,  $j = 3$  is a flat area, and  $j = 4$  is a polyline area.

Now substituting in (27) the expressions (19), (24) we get the formula for the resistance force to cutting the soil on the surface of the stony fraction:

$$P_s = \sum_{j=1}^{j=4} \left[ \pi R_s^2 \sin \frac{\mu}{2} \sum_{i=1}^{i=n} dE_i (\cos \theta_i + \cos \theta_{i+1}) \cos \xi_i \right] \tag{28}$$

$$P_g = 4\pi R_s^3 \gamma_s f_s / 3, \tag{29}$$

where  $\gamma_s$  is the volume weight of the stone, and  $f_s$  is the coefficient of friction of the stone with the ground.

Thus, on the basis of formulas (5) and (6), taking into account formulas (28) and (29), we obtain a generalized dependence for calculating the cutting resistance force of a soil containing a spherical stony particle under a steady state, with a sharp cutting element:

$$P_{rs} = \sqrt{(A - Hh_s / \sin \alpha)^2 + (A \tan \varphi_0)^2} \sin(\alpha + \varphi_0) b_s \eta_{sp_s} + \sum_{j=1}^{j=4} \left[ \pi R_s^2 \sin \frac{\mu}{2} \sum_{i=1}^{i=n} dE_i (\cos \theta_i + \cos \theta_{i+1}) \cos \xi_i \right] + 4\pi R_s^3 \gamma_s f_s / 3, \tag{30}$$

where  $i = 1, 2, 3 \dots n, j = 1 \dots 4, A = a_{\beta_i} (0,5 \gamma h_s^2 + p h_s + H h_s) / \sin \alpha$ .

The analysis of the dependence (30) shows that the cutting resistance force is influenced by the parameters of the strength and cutting of the soil, the radius of the stony fraction, and the influence of the latter on the cutting resistance force occurs according to a quadratic dependence.

When  $(h_s - R_s) < R_s$ , the rocky inclusion begins to protrude beyond the day surface of the ground, a “passive part” of the stone appears and the boundary of its appearance is determined by the critical value of  $\mu_{cr}$ :

$$\begin{cases} \mu_{cr} = a \sin \left[ \left( \frac{h_s - R_s}{R_s} \right) \right], \text{ when } (h_s - R_s) > 0 \\ \mu_{cr} = a \sin \left[ \left( \frac{R_s - h_s}{R_s} \right) \right], \text{ when } (h_s - R_s) < 0 \end{cases} \tag{31}$$

The change in the cutting width when the rocky inclusion protrudes beyond the cutting width is determined by the expression:

$$b_s = \begin{cases} b, \text{ when } b > 2 \cot(\psi_{ss}) (R_s / \cos \psi_{ss} - R_s) \\ 2 \cot(\psi_{ss}) (R_s / \cos \psi_{ss} - R_s), \text{ when } b \leq 2 \cot(\psi_{ss}) (R_s / \cos \psi_{ss} - R_s) \end{cases}, \tag{32}$$

where  $b$  is the cutting width of a homogeneous ground,  $\psi_{ss}$  – lateral angle of destruction.

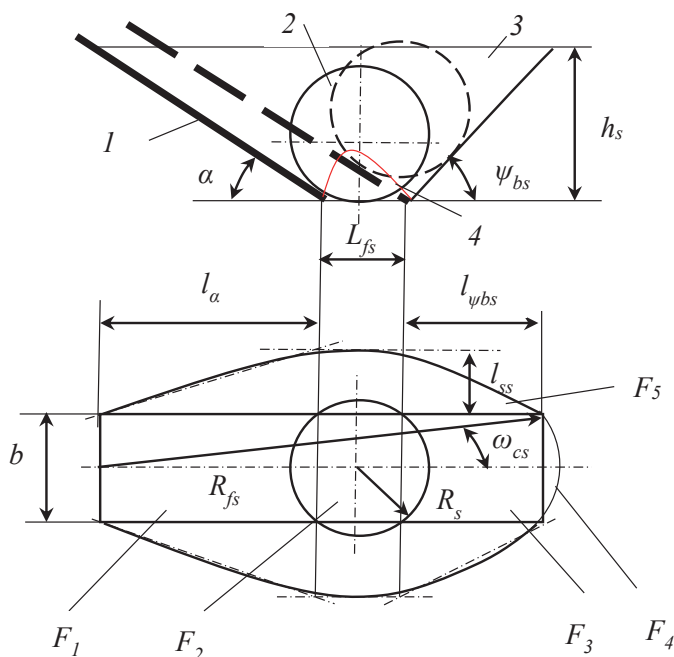


Fig. 3. Scheme for determining the volume of the destroyed soil with a rocky inclusion: 4 - graph of the change in the resistance force of the soil,  $L_{fs}$  - cutting length (experimentally determined),  $F_1, F_2, F_3, F_4, F_5$  - the area of the parts of the destruction.

The volume of the soil of the central part and the lateral destruction (Fig.3) calculated by the formulas:

$$V_1 = h_s [0.5h_s \cot(\alpha)b_s], V_2 = h_s(L_{fs}b_s), V_3 = h_s [0.5h_s \cot(\psi_{bs})b_s], V_4 = h_s [0.25R_{fs}^2(2\omega_{cs} - \sin 2\omega_{cs})],$$

$$\omega_{cs} = a \sin(0.5b_s R_{fs}), V_5 = h_s [(0.5R_{fs} \cos \omega_{cs} + L_{fs}) \cdot l_{ss}/3], R_{fs} = \sqrt{\{h_s [(\cot(\alpha) + \cot(\psi_{bs})) + L_{fs}]^2 + (0.5b_s)^2\}} \quad (33)$$

The spatiality coefficient is defined by the expression:

$$\eta_{sp} = 1 + n_s V_s / \left( \sum_{i=1}^5 V_i \right) \quad (34)$$

where  $n_s = 0$ , with the free cutting,  $n_s = 1$ , when polubarinova cutting,  $n_s = 2$ , at locked cuts.

Experimental work was carried out on cutting the soil containing stony fractions on the stand [25] (Table 1).

Table 1 - Initial experience data

Ground	$R_s, m$	$C, Pa$	$\gamma, n/m^3$	$\varphi, ^\circ$	$\varphi_0, ^\circ$	$\alpha, ^\circ$	$h, m$	$b, m$	$\eta_{sps}$	$k_h$
Sandy loam	0.064-0.108	15500	19790	32	25.6	45	0.15	0.15	1.589...1.663	0...0.05

Figure 4 shows a comparative analysis of the cutting resistance force according to different methods and experiments.

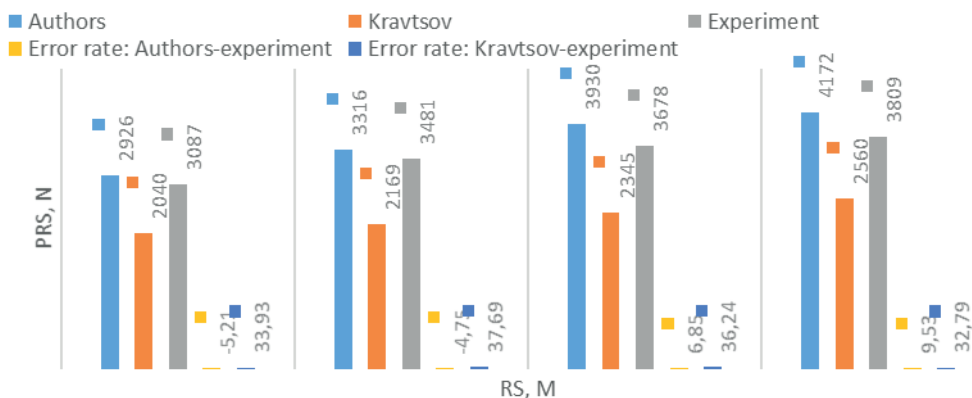


Fig. 4. Comparative analysis of the cutting resistance force according to different methods and experiments

The analysis shows that the discrepancy between the values of the resistance force according to the methods of the authors and E. Kravtsov is in the range of 30.29...40.32% when cutting sandy loam containing stony fractions with an average radius of 0.064...0.108 m. Obviously, this error is caused by the fact that in [11] the cutting resistance force is determined without taking into account the effects of the angle, cutting width, features of the normal and tangent components of the passive ground pressure of a homogeneous and rocky zone. The error in the forces of resistance to cutting soil with stones between the authors' method and the experiment is -5.21...9.53%, and between the method of E. Kravtsov and the results of the experiment – 32.79...37.69%. It should be noted that the discrepancy in the resistance force determined by the E. Kravtsov method increases with the increase in the size of the stone and the cohesive properties of the soil.

According to the data [9], a computational analysis of the dynamics of the cutting resistance force as a function of the number of stony fractions in the ground is carried out (Fig.5).

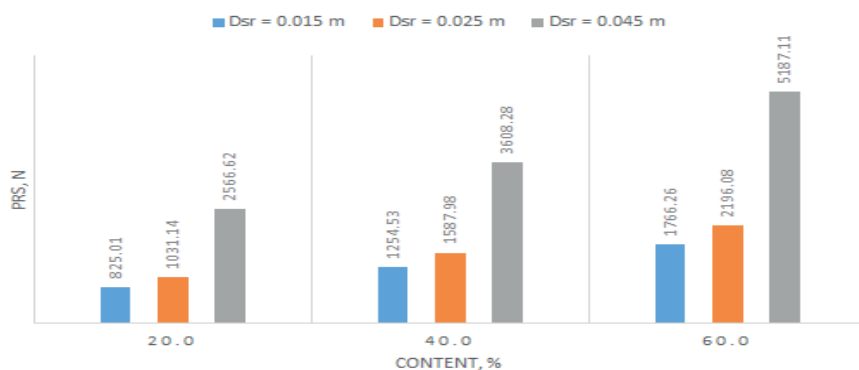


Fig. 5. Computational analysis of the change in the cutting resistance force depending on the content of stones in the ground

The analysis shows that with an increase in the content of stones, the values of the angle of friction of the soil against the metal and the specific cohesion of the soil increase, which ultimately cause an increase in the resistance force of the soil-stone mixture to cutting.

**Conclusions the perspective of the methodology application.** Soils containing stony fractions can be represented as a mechanical mixture of soil with stony fractions. The heterogeneity and random content of rock particles in the soil have a significant impact on the strength of the soil resistance to cutting. The similarity principles allow the use of passive ground pressure components to develop an analytical model of the cutting resistance force. Its value is influenced by the strength parameters and the cutting geometry of the soil, the radius of the stony fractions and the structural effect. The influence of the radius of the stone on the strength of the resistance to cutting occurs according to the quadratic dependence.

The above methodology allows us to develop methods for predicting the strength of resistance to cutting of soil containing stony fractions of ellipsoid geometric shape.

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## ТАСТЫ ҚОСЫЛЫСЫ БАР ТОПЫРАҚТЫ КЕСУГЕ ҚАРСЫ ТҮРУ КҮШІН БОЛЖАУ

**Аннотация.** Тасты қоспалары бар топырақтарды әр түрлі тасты қоспалары бар топырақ-тас қоспасы ретінде қарастыруға болады. Бұл механикалық қоспа құрамы негізінен кездейсоқ жүктемелердің пайда болуы жер қазу машинасының металл конструкциясының мерзімінен бұрын істен шығуынан, оны пайдалану кезінде топырақтың құнын арттыруы болып табылады.

Тасты қосындылары бар топырақты кесуге қарсы тұру мақсатында болжау үшін борпылдақ ортаның шекті тепе-теңдік теориясы, тәуелділік параметрлерін өлшемді талдау негізінде аналитикалық модель жасалды. Кесу элементінің әрекет ету координатына байланысты тасты қосылумен топырақтың бұзылу түрлері анықталды. Тәжірибелік мәліметтермен салыстырғанда әзірленген модельдің дұрыстығы бағаланды. Гетерогенділіктен және тасты қосындылары бар топырақтың құрылымдық апатынан туындайтын кесуге қарсылық күшінің динамикасына есептеу талдауы жүргізілді.

**Түйінді сөздер:** Тасты қосу, тасты қосу кесуге төзімділік күші, аналитикалық модель, борпылдақ ортаның шекті тепе-теңдік теориясы, өлшемді талдау, құрылымдық күй, гетерогенділік, құрылымдық апат бар топырақ.

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## ПРОГНОЗИРОВАНИЕ СИЛЫ СОПРОТИВЛЕНИЯ РЕЗАНИЮ ГРУНТА СОДЕРЖАЩЕГО КАМЕНИСТОЕ ВКЛЮЧЕНИЕ

**Аннотация.** Грунты, содержащие каменные включения можно представить, как грунтово-каменную смесь с разным содержанием каменных включений. Указанная механическая смесь главным образом является источником генерации случайных нагружений и преждевременных усталостных разрушений металлоконструкции землеройной машины в процессе ее эксплуатации, повышения себестоимости разработки грунтов.

С целью прогнозирования силы сопротивления резанию грунта, содержащего каменные включения разработана аналитическая модель на основе теории предельного равновесия сыпучей среды, размерного анализа параметров зависимости. Установлены виды разрушения грунта с каменным включением в зависимости от координаты действия режущего элемента. Оценена корректность разработанной модели в сопоставлении с экспериментальными данными. Проведен вычислительный анализ динамики силы сопротивления резанию возникающей от неоднородности и структурной случайности грунтов, содержащих каменные включения.

**Ключевые слова:** грунт, содержащий каменное включение, каменное включение, сила сопротивления резанию, аналитическая модель, теория предельного равновесия сыпучей среды, размерный анализ, структурное состояние, неоднородность, структурная случайность.

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## МАЗМҮНЫ-СОДЕРЖАНИЕ-CONTENTS

<b>Abuova R.Zh., Ten E.B., Burshukova G.A.</b> STUDY OF VIBRATION PROPERTIES OF CERAMIC-METAL NANOSTRUCTURAL TIN-CU COATINGS WITH DIFFERENT COPPER CONTENT 7 AND 14 AT. % ON CHROMIUM-NICKEL-VANADIUM STEELS.....	6
<b>Abetov A., Kudaibergenova S.</b> INTEGRATED RESEARCH OF SUFFOSION AND KARST PROCESSES AT THE KOGCF BY GEOLOGICAL AND GEOPHYSICAL AND GEODESIC METHODS.....	14
<b>Amangeldykyzy A., Kopobayeva A.N., Bakyt A., Ozhigin D.S., Blyalova G.G.</b> MINERALOGY AND GEOCHEMISTRY OF THE SHUBARKOL DEPOSIT JURASSIC COALS.....	23
<b>Dikanbayeva A.K., Auyeshov A.P., Satayev M.S., Arynov K.T., Yeskibayeva Ch.Z.</b> RESEARCHING OF SULFURIC ACID LEACHING OF MAGNESIUM FROM SERPENTINES.....	32
<b>Duisen G.M., Aitzhanova D.A.</b> NATURAL RESOURCE POTENTIAL OF KAZAKHSTAN AND CENTRAL ASIAN COUNTRIES: PROSPECTS OF USE.....	39
<b>Edygenov E.K., Vassin K.A.</b> ELECTROMAGNETIC VEHICLE WITH AUTOMATED CONTROL SYSTEM FOR SURFACE MINING OPERATIONS.....	47
<b>Ismailov B.A., Dossaliev K.S.</b> TECHNOLOGICAL REGULATIONS OF CONDITIONS IN PRODUCTION OF FERTILIZER MIXTURES “ZHAMB-70”.....	54
<b>Issagaliyeva A.K., Istekova S.A., Aliakbar M.M.</b> GEOPHYSICAL DATA COMPLEX INTERPRETATION TECHNIQUES FOR STUDIES OF THE EARTH CRUST DEEP HORIZONS IN THE NORTH CASPIAN REGION.....	61
<b>Mekhtiyev A.D., Soldatov A.I., Neshina Y.G., Alkina A.D., Madi P.Sh.</b> THE WORKING ROOF ROCK MASSIF DISPLACEMENT CONTROL SYSTEM.....	68
<b>Mustafayev Zh.S., Kozykeeva A.T., Tursynbayev N.A., Kireychev L.V.</b> APPLIED MODEL OF ENVIRONMENTAL SERVICES - DEVELOPMENT OF ECOLOGICAL AND ECONOMIC DRAINAGE SYSTEM OF TRANSBOUNDARY RIVER BASINS (on the example of the Talas river basin).....	77
<b>Petr Hajek, Baimaganbetov R.S.</b> GEOSTABILIZATION OF ECOLOGICAL EQUILIBRIUM AS A RESULT OF FOREST FIRES.....	84
<b>Salikhov N.M., Pak G.D., Shepetov A.L., Zhukov V.V., Seifullina B.B.</b> HARDWARE-SOFTWARE COMPLEX FOR THE TELLURIC CURRENT INVESTIGATION IN A SEISMICALLY HAZARDOUS REGION OF ZAILIYSKY ALATAU.....	94

<b>Saukhimov A.A., Ceylan O., Baimakhanov O.D., Shokolakova Sh.K.</b> REDUCING POWER AND VOLTAGE LOSSES IN ELECTRIC NETWORKS OF OIL FIELDS USING THE MOTH FLAME OPTIMIZATION ALGORITHM.....	103
<b>Soltanbekova K.A., Assilbekov B.K., Zolotukhin A.B., Akasheva Zh.K., Bolysbek D.A.</b> RESULTS OF LABORATORY STUDIES OF ACID TREATMENT OF LOW-PERMEABILITY ROCK CORES.....	113
<b>Surimbayev B., Bolotova L., Shalgymbayev S., Razhan E.</b> RESEARCH OF THE COMPLEX STAGE-BY-STAGE SCHEME OF GRAVITY SEPARATION OF GOLD ORE.....	124
<b>Temirbekov N.M., Los V.L., Baigereyev D.R., Temirbekova L.N.</b> MODULE OF THE GEOINFORMATION SYSTEM FOR ANALYSIS OF GEOCHEMICAL FIELDS BASED ON MATHEMATICAL MODELING AND DIGITAL PREDICTION METHODS.....	137
<b>Tileuberdi N., Zholtayev G.ZH., Abdeli D. Zh., Ozdoev S.M.</b> INVESTIGATION OF DRAINAGE MECHANISM OF OIL FROM PORES OF OIL SATURATED ROCKS USING NITROGEN AT THE LABORATORY CONDITION.....	146
<b>Tleulesov A.K., Suyundikov M.M., Shomanova Zh.K., Akramov M.B., Suiindik N.M.</b> ASSESSMENT OF QUALITATIVE AND QUANTITATIVE ELEMENTAL COMPOSITION OF WASTE IN THE TERRITORY OF SLUDGE COLLECTOR OF PAVLODAR ALUMINIUM PLANT.....	153
<b>Turgumbayev J.J., Turgunbayev M.S.</b> PREDICTION OF THE CUTTING RESISTANCE FORCE OF THE SOIL CONTAINING STONY FRACTIONS.....	161
<b>Uakhitova B., Ramatullaeva L., Imangazin M., Taizhigitova M., Uakhitov R.</b> ON THE STATE OF INDUSTRIAL INJURIES OF WORKERS IN INDUSTRIAL ENTERPRISES OF THE AKTUBINSK REGION.....	170
<b>Sherov K.T., Sikhimbayev M.R., Absadykov B.N., Karsakova N.Zh. Myrzakhmet B.</b> METROLOGICAL ENSURING ACCURACY OF MEASUREMENT OF ANGLES V-SHAPED SURFACES GUIDE PARTS OF MACHINES FOR PETROCHEMICAL AND GEOLOGICAL EXPLORATION INDUSTRY.....	176



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